# Space truss design using STAAD.Pro Software

## Basil Baby<sup>1</sup>, Biya S Lakshmi<sup>2</sup>, Chandni Narayani, Deepak Salilan<sup>4</sup>, Merin Mathews<sup>5</sup>

<sup>1234</sup>BTech graduate students, Dept. of Civil Engineering, Mar Athanasius College of Engineering, Kothamangalam, Kerala, India.

<sup>5</sup>Asst.Professor, Dept. of Civil Engineering, Mar Athanasius College of Engineering, Kothamangalam, Kerala, India \*\*\*

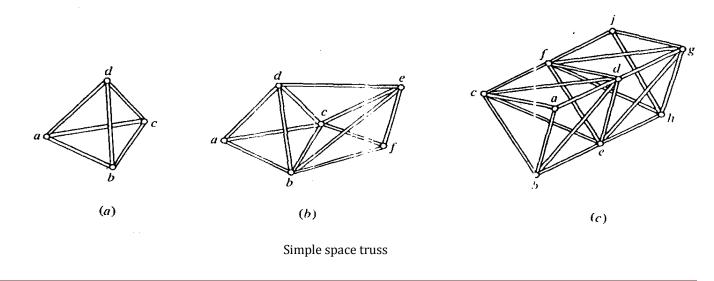
**Abstract** - The search for new structural forms to accommodate large unobstructed areas has always been the main objective of architects and engineers. With the advent of new building techniques and construction materials, spaceframesfrequently provide the right answer and satisfy the requirements for lightness, economy, and speedy construction. In the past few decades, the proliferation of the space frame was mainly due to its great structural potential and visual beauty. New and imaginative applications of space frames are being demonstrated in the total range of building types, such as sports arenas, exhibition pavilions, assembly halls, transportation terminals, airplane hangars, workshops, and warehouses. They have been used not only on long-span roofs, but also on mid- and short-span enclosures as roofs, floors, exterior walls and canopies. Here design and analysis of a space truss using STAAD.Pro software has been done as an example.

## **1. INTRODUCTION**

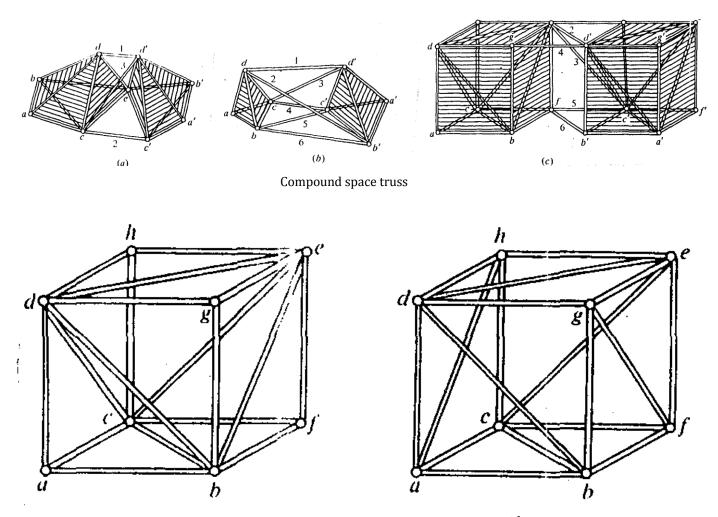
A space frame is a structure system assembled of linear elements so arranged that forces are transferred in a three-dimensional manner. In some cases, the constituent element may be two-dimensional. Macroscopically a space frame often takes the form of a flat or curved surface. Occasionally the term space truss appears in the technical literature. According to the structural analysis approach, a space frame is analyzed by assuming rigid joints that cause internal torsions and moments in the members, whereas a space truss is assumed as hinged joints and therefore has no internal member moments. The choice between space frame and space truss action is mainly determined by the joint-connection detailing and the member geometry is no different for both. However, in engineering practice, there is no absolutely rigid or hinged joints. For example, a double layer flat surface space frame is usually analyzed as hinged connections, while a single layer curved surface space frame may be analyzed either as hinged or rigid connections. The term *space frame* will be used to refer to both space frames and space trusses. Space truss is commonly used in three dimensional structural element. The forces are subjected axially in space truss elements, which are assumed pin connected where all the loads act only at joints. Due to application of forces, deformation happens in the axial direction and space trusses cannot sustain the shear and moment

## **1.1 Types of space trusses**

The commonly used space truss elements can be broadly classified into three types viz. Simple, Compound and Complex. Simple space is made from tetrahedron. This can be enlarged by adding members. Compound space truss is made by joining simple space truss elements. Complex space truss doesn't belong to the above categories.







Complex space truss

## 1.2 Determinacy and Stability aspects of space truss

For internal determinacy, if m = number of members; j = number of joints; r = number of supports;

If m=3j+r then the truss is stable and internally determinate

If m<3j+r then it is unstable and

If m>3j+r the truss is internally indeterminate

Internal stability can sometimes be checked by careful inspection of the member arrangement.

External redundancy can be determined in terms of r

## Externally

r < 6 Unstable Truss</pre>

*r* = 6 Determinate if Truss is Stable

*r* > 6 Indeterminate Truss

The assumptions for the design are

1. The members are joined together by smooth pins (no friction - cannot resist moment)

2.All loadings and reactions are applied centrally at the joints

3. The centroid for each members are straight and concurrent at a joint. Therefore, each truss member acts as an axial force member:

If the force tends to elongate> Tensile (T)

If the force tends to shorten >Compressive (C)

## 2. DESIGN OF A SPACE TRUSS USING STAAD.Pro

STAAD or Structural analysis and Design software application is a widely used software for the purpose of analysis and design of civil engineering structures. It supports over 90 international steel, concrete timber and Aluminum design codes. It can make use of various forms of analysis to more recent analysis like p-delta analysis, geometric nonlinear analysis. It can also make use of various forms of dynamic analysis methods from time history analysis to response spectrum analysis.

For the analysis of a structure in STAAD.Pro one has to illustrate the structure in it first. Consequently the entire loads are to be applied. Different loads applicable to the structure are predefined in the software. The loads are separately provided. Different load combinations if any, should be provided. The software gives the Bending moment diagram, Shear force diagram and the deflection profile of the structure.

Here, an example of analysis of a space trussed roof using the software is illustrated.

SPACE TRUSSED ROOF:

Shape: Curved (Arch) Span: 21.5 m Length: 52 m

Rise of the arch shape: 3.5m

One can draw the shape directly into the software or may be imported after drawing in CAD software. Next step is to apply loads. It has been found that there are 3 kinds of loads acting in the truss viz.

1. Dead load due to the weight of the roofing material and the self-weight of the truss itself

2. Live load in case of labor entrance to top of roofing for the construction or maintenance whose value is to be provided as per the design code

3. Wind load

### **2.1 LOAD CALCULATIONS**

1. Dead load as per the code provision of roofing material = 0.762 kN

2. Live load= 0.106 kN

3. Wind load calculations

Basic Wind Speed in the region = 39 m/s Design wind speed = 39.39 m/s Design wind pressure = 930 N/m<sup>2</sup> Wind pressure coefficients Internal coefficients, Cpi =  $\pm$  0.2 External coefficients for ached roof At  $\theta$  = 0° Cpe1 = -0.861

Cpe2 = +0.22

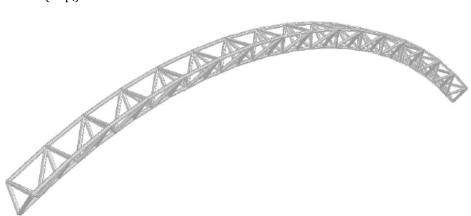
At 
$$\theta = 90^{\circ}$$

Near gable end = -0.5

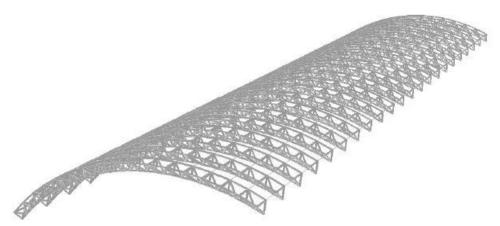
Near middle half = -0.1

Wind force at X-direction  $(-Cp_i) = 1.776 \text{ kN}$ Wind force at X-direction  $(+Cp_i) = 1.106 \text{ kN}$ Wind force at Z-direction  $(-Cp_i) = 1.506 \text{ kN}$ 

Wind force at Z-direction (  $Cp_i$ ) = 1.800 kV Wind force at Z-direction (+ $Cp_i$ ) = 0.837 kN



Rendered view of space truss



Rendered view of roof

## STAAD.Pro Input data:

Structure Type SPACE FRAME

Number of Nodes	67	Highest Node	1409
Number of Elements	179	Highest Beam	3761

Number of Basic Load Cases	7
Number of Combination Load Cases	10

## **Section Properties**

Prop	Section	Area	l <sub>yy</sub>	l <sub>zz</sub>	J	Material		
		(mm <sup>2</sup> )	(mm⁴)	(mm⁴)	(mm⁴)			
1	PIPS40	1.92E+3	2.84E+6	2.84E+6	5.68E+6	STEEL		
2	PIPS80	5.06E+3	28.3E+6	28.3E+6	56.7E+6	STEEL		
3	PIPS35	1.62E+3	1.88E+6	1.88E+6	3.76E+6	STEEL		
4	PIPS30	1.34E+3	1.19E+6	1.19E+6	2.37E+6	STEEL		
5	PIPS20	645.160	261E+3	261E+3	522E+3	STEEL		
6	PIPS25	1.03E+3	604E+3	604E+3	1.2E+6	STEEL		
7	PIPX20	896.772	344E+3	344E+3	688E+3	STEEL		
8	PIPX15	645.160	155E+3	155E+3	310E+3	STEEL		
9	PIPS15	483.870	122E+3	122E+3	244E+3	STEEL		
10	PIPS12	399.999	76.6E+3	76.6E+3	153E+3	STEEL		
11	PIPX10	387.096	42E+3	42E+3	84E+3	STEEL		
12	PIPS10	296.774	34.5E+3	34.5E+3	69.2E+3	STEEL		
13	PIPX7	264.516	17.9E+3	17.9E+3	35.7E+3	STEEL		
14	PIPS60	3.37E+3	11E+6	11E+6	22E+6	STEEL		

### DESIGN RESULTS: STAAD Output Summary

## PROBLEM STATISTICS

#### -----

NUMBER OF JOINTS	67	NUMBER OF MEMBERS	179
NUMBER OF PLATES	0	NUMBER OF SOLIDS	0
NUMBER OF SURFACES	0	NUMBER OF SUPPORTS	2



#### ALL UNITS ARE - KN METE (UNLESS OTHERWISE Noted)

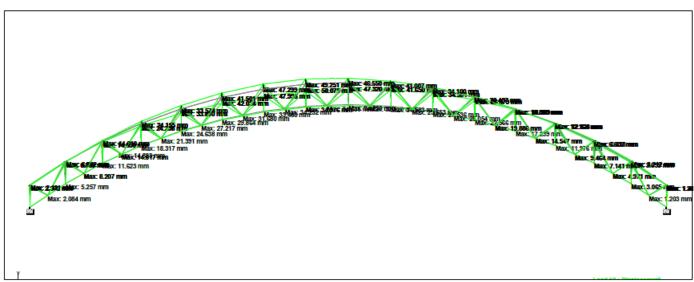
			FX	MY	MZ	LOADING/ LOCATION
3722	ST	PIPS30		(AISC SECTIO	ONS)	
			PASS	IS-7.1.2	0.620	12
			6.52 T	0.00	2.59	1.45
3723	ST	PIPX20		(AISC SECTION	ONS)	
			PASS	IS-7.1.1(A)	0.894	16
			17.63 C	0.00	1.39	0.00
3724	ST	PIPS25		(AISC SECTION	ONS)	
			PASS	IS-7.1.1(A)	0.774	16
			0.34 C	0.00	2.03	0.00
3725	ST	PIPS25		(AISC SECTION	ONS)	
			PASS	7.1.2 BEND C	0.758	12
			3.36 T	0.00	1.99	0.00
3726	ST	PIPS15		(AISC SECTION		
			PASS			
			1.15 T	0.00	0.59	0.00
3727	ST	PIPS15		(AISC SECTION	ONS)	
			PASS			
			1.44 C	0.00	0.62	0.96
3729	ST	PIPX20		(AISC SECTION	/	
				IS-7.1.1(B)		
			64.40 C	0.00	0.58	0.00
3730	ST	PIPX20		(AISC SECTION		
			PASS			12
			1.51 T	0.00		1.38
3731	ST	PIPS25		(AISC SECTION		
				IS-7.1.1(A)		
			6.86 C	0.00	1.65	1.38

### STEEL TAKE-OFF

\_\_\_\_\_

1	PROFILE		LENGTH (METE)						WEI	GHT	(KN)	)							
181.	. 3620	то	3628	3630	то	3639	3641	то	3650	3652	то	3661	3663	то	3672	3674	то	3683	3 -
182.	. 3685	TO	3694	3696	то	3705	3707	то	3716	3718	то	3727	3729	то	3738	-			
183	. 3740	TO	3749	3751	то	3760													
ST	PIPS80	D					3.	37			1.	.312							
ST	PIPS40	D					з.	82			0.	562							
ST	PIPS35	5					4.	01			Ο.	499							
ST	PIPS30	D					41.	50			4.	278							
ST	PIPS20	D					10.77 0.534				.534								
ST	PIPS25	5					36.	36.44 2.872				872							
ST	PIPX20	D					28.82 1.985					985							
ST	PIPX15	5					15.13 0.750					750							
ST	PIPS15	5					9.60			0.357									
ST	PIPS12	2		11			11.	.1.30 0.347											
ST	PIPX10	D					4.	57			0.	136							
ST	PIPS10	D		6.25			25			0.	.143								
ST	PIPX7			0.96					0.020										
ST	PIPS60	D		1.92				0.497											
				TOTAL =						14.	291								





Whole Structure Displacements 0.0508mm:1mm 16 COMBINATION LOAD CASE 16 (Input data was modified after picture taken)

## **3. CONCLUSIONS**

Space truss has many advantages over the other types on account of its light weight, mass production, stiffness and versatility. Moreover it is very much preferable in case of long spans when compared to others. On account of the simplicity and aesthetic aspects the arch shape is preferable. Analysis of civil engineering structures become much easier with the advent of the STAAD Pro. Software. STAAD outputs the Bending moment diagram shear force diagram and deflection profile of the structure. STAAD not only check whether the structure is safe or not for the given loads but also advises suitable dimensions in order to make the structure safe for the given loading conditions.

## **4. REFERENCES**

[1] Bureau of Indian Standards, IS 875 (Part 1, 2, 3, 5): 1987 – Code of Practice for Design Load (Other than Earthquake) for Buildings and Structures

Authors:



Basil Baby BTech Graduate Student



Biya S Lakshmi BTech Graduate Student



Chandni Narayani BTech Graduate Student



Deepak Salilan BTech Graduate Student